

CITY OF TACOMA

Public Work Engineering Division

ADDENDUM NO. 1 DATE: 3/4/2022

REVISIONS TO:

Request for Bids Specification No. PW22-0059F 15th Street Transient Moorage Dock Replacement

NOTICE TO ALL BIDDERS:

This addendum is issued to clarify, revise, add to or delete from, the original specification documents for the above project. This addendum, as integrated with the original specification documents, shall form the specification documents. The noted revisions shall take precedence over previously issued specification documents and shall become part of this contract.

REVISIONS TO THE SUBMITTAL DEADLINE:

The submittal deadline remains the same.

REVISION TO THE PLANS:

Add the following plans sheets 1 thru 14.

NOTE: Acknowledge receipt of this addendum by initialing the corresponding space as indicated on the signature page. Vendors who have already submitted their bid/proposal may contact the Purchasing Division at 253-502-8468 and request return of their bid/proposal for acknowledgment and re-submittal. Or, a letter acknowledging receipt of this addendum may be submitted in an envelope marked Request for Bids Specification No. PW22-0059F Addendum No. 1. The City reserves the right to reject any and all bids, including, in certain circumstances, for failure to appropriately acknowledge this addendum.

cc: Darius Thompson, Public Works Engineering

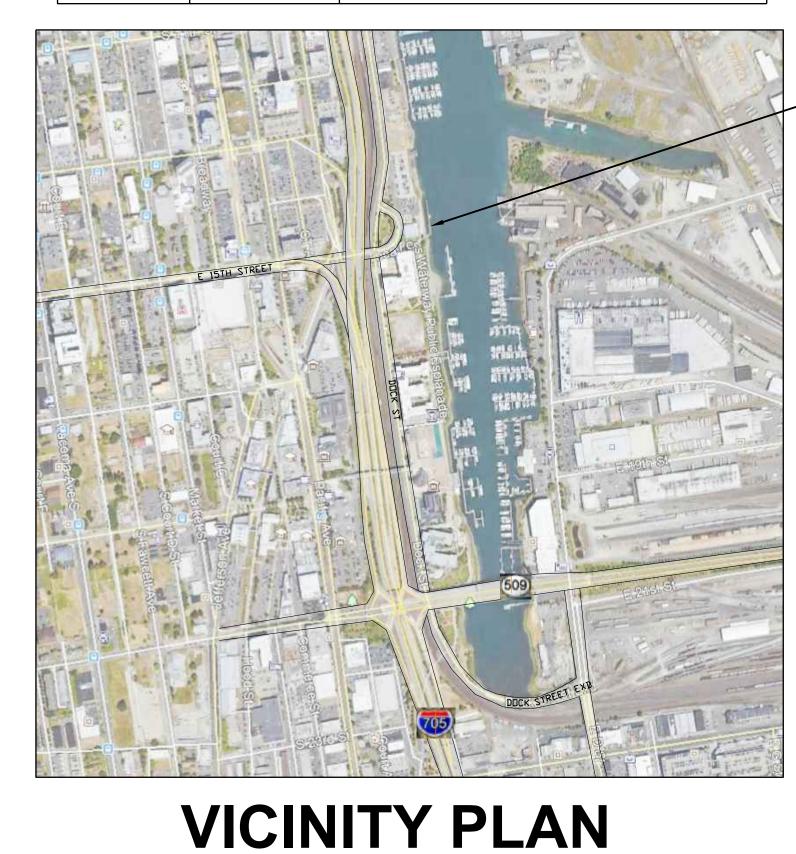
Form No. SPEC-220A Revised: 07/30/2015

15th STREET TRANSIENT MOORAGE DOCK REPLACEMENT

FOR THE CITY OF TACOMA, WASHINGTON

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SITE PLAN

THE FISH PEDDLER

FOR CONSTRUCTION

100% SUBMITTAL

E 15TH STREET

DEPARTMENT OF PUBLIC WORKS Chris Larson

15TH ST TRANSIENT MOORAGE DOCK REPLACEMENT

PW-00000-00-0² S1

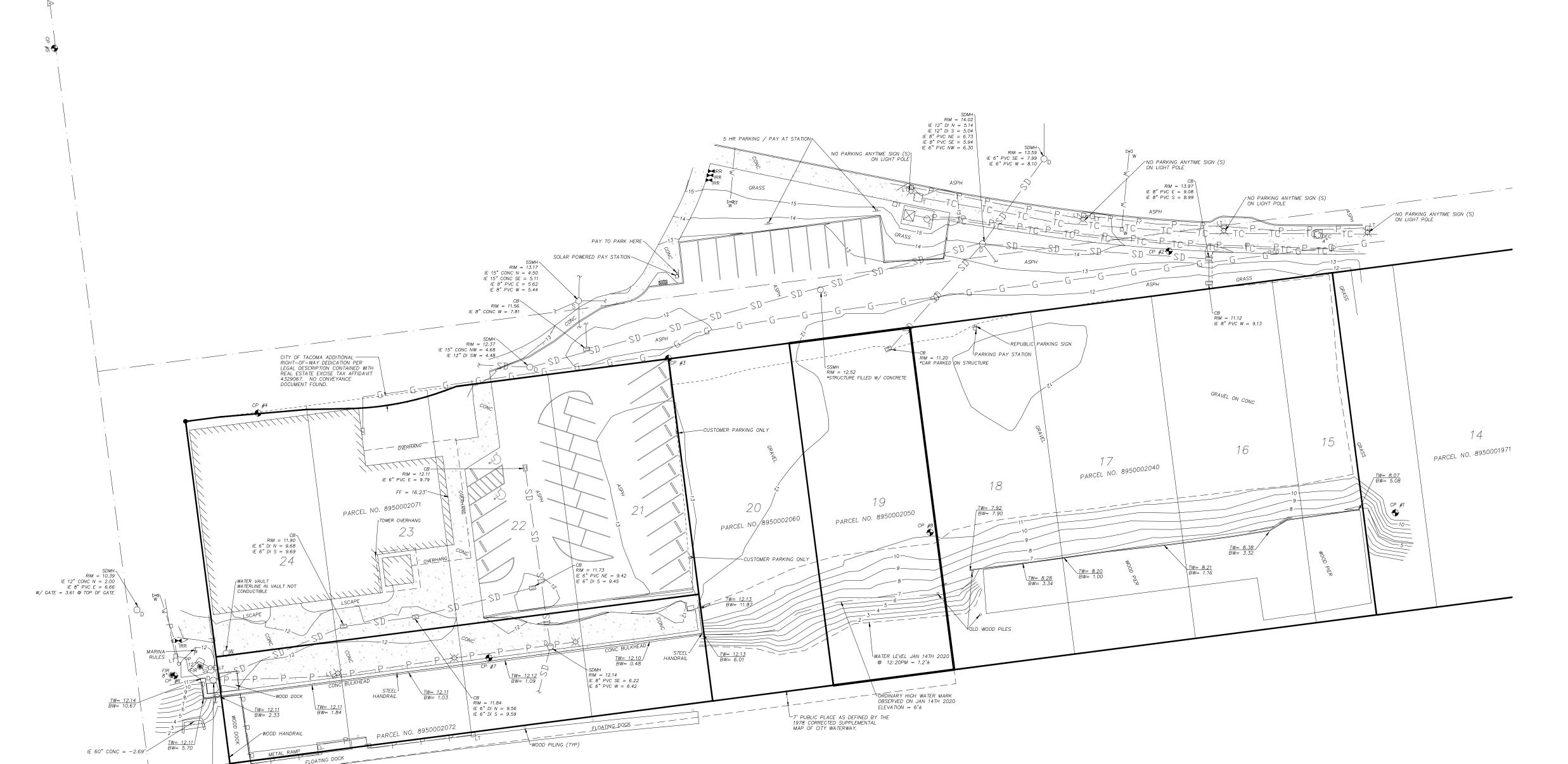
- MOORAGE DOCK

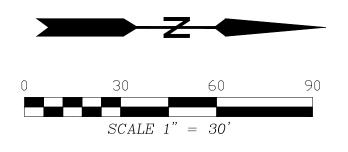
COVER SHEET

CITY OF TACOMA

TOPOGRAPHIC SURVEY

SECTION 4, TOWNSHIP 20N, RANGE 3E, W.M. CITY OF TACOMA, PIERCE COUNTY, WASHINGTON





	LEGEND				
•	FOUND REBAR AND CAP FOUND STREET MONUMENT	⊙ _D	STORM DRAIN MAN HOLE		
⊕ ⊠	CONTROL POINTS TRANSFORMER	∴	PILE SIGN AS NOTED		
⊡P ⊡LT	POWER METER LIGHT JUNCTION BOX	₩SPECIES ₩SIZE SPECIES SIZE	CONIFER AS NOTED DECIDUOUS AS NOTED		
-⊙-sv ××	POWER POLE LIGHT POLE ANCHOR		WHEEL STOP BUILDING OVERHANG FENCE AS NOTED		
⊙ _T	TELECOMMUNICATIONS MANHOLE TELECOMMUNICATIONS VAULT		EDGE OF PAVEMENT EDGE OF GRAVEL SANITARY SEWER		
Ø □1	TELECOMMUNICATIONS PEDESTAL FIRE HYDRANT	G	STORM DRAIN GAS LINE WATER LINE		
⊡W W W	WATER METER WATER VALVE IRRIGATION VALVE	— P — — — — — — — — — — — — — — — — — —	UNDERGROUND POWER UNDERGROUND TELECOMMUNICATIONS		
ÍRR ⊙W □G	WATER MANHOLE GAS METER		TRUNCATED DOME HATCH BUILDING HATCH		
⊙ °S	SANITARY SEWER MAN HOLE SANITARY SEWER CLEAN OUT	· · · · · · · · · · · · · · · · · · ·	CONCRETE HATCH		

BASIS OF MERIDIAN

WASHINGTON STATE PLANE COORDINATES NAD 83/91, SOUTH ZONE 4602, PER WSDOT CONTROL POINT NUMBERS GP27509-117 AND GP27509-9.

UNDERGROUND UTILITY LOCATES SHOWN HEREON WERE PER ON SITE LOCATES PERFORMED BY OTHERS, KPFF DID NOT CALL ON UTILITY LOCATES FOR THE PURPOSES OF THIS SURVEY.

BOUNDARY NOTE
BOUNDARY AND RIGHT OF WAY SHOWN HEREON ARE PER BEST
AVAILABLE RECORD INFORMATION, A TITLE REPORT WAS NOT OBTAINED
AND NO EASEMENTS ARE SHOWN BY THIS SURVEY.

METHOD OF SURVEY

CONTROL PERFORMED USING TOPCON GR5 GPS RECEIVERS,

TOPOGRAPHIC SURVEY PERFORMED USING CONVENTIONAL FIELD

TRAVERSE METHODS WITH THE USE OF A TOPCON PS 3" POWER STATION.

THE WORK PERFORMED DURING THE COURSE OF THIS SURVEY MEETS OR EXCEEDS THE STANDARDS AS SET FORTH IN WAC 332-130-090. COMPLETED IN JANUARY 2020.

100% SUBMITTAL









		CONSTRUCTION CHECKED
		ВҮ
		DATE
		FIELD BOOKS
DATE	APPD	

2/4/22

DESIGNED

DRAWN

SLC

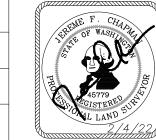
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1900152-02

PROJECT NAME





DocuSigned by:

Chris Larson



15TH ST TRANSIENT MOORAGE DOCK REPLACEMENT SITE SURVEY

PW-00000-00-02 SHEET NO. S2

SHEET 02 OF 14

THESE NOTES CONTAIN GENERAL INFORMATION AND ARE NOT COMPLETE FOR CONSTRUCTION PURPOSES. REFERENCE SPECIAL PROVISIONS AND WSDOT STANDARD SPECIFICATIONS. CONTRACTOR SHALL VERIFY INFORMATION GIVEN HERE AND BRING ANY CONFLICTS TO THE ATTENTION OF THE ENGINEER BEFORE BEGINNING AFFECTED WORK

DESIGN LOADS

ALL DESIGN AND CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF THE INTERNATIONAL BUILDING CODE (IBC), 2018 EDITION AS AMENDED BY THE CITY OF TACOMA

GENERAL NOTES

SUBMITTALS:

SHOP DRAWINGS AND MATERIAL DATA SHALL BE SUBMITTED TO THE ENGINEER PRIOR TO ANY FABRICATION OR CONSTRUCTION FOR THE FOLLOWING ITEMS: PILING, BEAM AND STRINGER HANGER FABRICATIONS, ANCHORAGES, TIMBERS, CROSS BRACING SYSTEM, STEEL RAILING SYSTEM, BIDDER DESIGNED GANGWAY AND GANGWAY SUPPORT CONNECTIONS, BIDDER DESIGNED MOORAGE FLOATS AND BIDDER DESIGNED PILE GUIDE COLLARS.

INSPECTION:

SPECIAL INSPECTION, PER IBC CHAPTER 17 WILL BE PERFORMED BY AN APPROVED TESTING AGENCY AS OUTLINED IN THE SPECIAL INSPECTION SCHEDULE.

SPECIAL CONDITIONS:

CONTRACTOR SHALL VERIFY ALL LEVELS, DIMENSIONS, AND EXISTING CONDITIONS IN THE FIELD BEFORE PROCEEDING. CONTRACTOR SHALL NOTIFY THE ENGINEER OF ANY DISCREPANCIES OR FIELD CHANGES PRIOR TO INSTALLATION OR FABRICATION. IN CASE OF DISCREPANCIES BETWEEN THE EXISTING CONDITIONS AND THE DRAWINGS. THE CONTRACTOR SHALL OBTAIN DIRECTION FROM THE ENGINEER BEFORE PROCEEDING. DIMENSIONS NOTED AS PLUS OR MINUS (\pm) OR (REF) INDICATE UNVERIFIED DIMENSIONS AND ARE APPROXIMATE. NOTIFY THE ENGINEER IMMEDIATELY OF CONFLICTS OR EXCESSIVE VARIATIONS FROM INDICATED DIMENSIONS. NOTED DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS -- DO NOT SCALE DRAWINGS. DIMENSIONS OF EXISTING CONDITIONS MAY BE BASED ON RECORD DRAWINGS OR FIELD OBSERVATIONS AND ARE TO BE FIELD-VERIFIED BY THE CONTRACTOR.

CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS BEFORE COMMENCING ANY DEMOLITION. CONTRACTOR SHALL PROVIDE ADEQUATE SHORING AND BRACING OF ALL STRUCTURAL MEMBERS, EXISTING CONSTRUCTION AND SOIL EXCAVATIONS, AS REQUIRED, AND IN A MANNER SUITABLE TO THE WORK SEQUENCE. TEMPORARY SHORING AND BRACING SHALL NOT BE REMOVED UNTIL ALL FINAL CONNECTIONS HAVE BEEN COMPLETED IN ACCORDANCE WITH THE DRAWINGS AND MATERIALS HAVE ACHIEVED DESIGN STRENGTH.

TIMBER

- 1. EACH PIECE OF LUMBER SHALL BEAR A STAMP INDICATING A GRADE MARK OF THE WEST COAST LUMBER INSPECTION BUREAU (WCLIB), WESTERN WOOD PRODUCTS ASSOCIATION (WWPA), OR OTHER AGENCY ACCREDITED BY THE AMERICAN LUMBER STANDARD COMMITTEE (ALSC).
- 2. STRUCTURAL GLUED LAMINATED TIMBER SHALL BE DF/DF 24F-V8.
- 3. DECK TIMBERS SHALL BE HEM FIR #2 OR BTR.
- 4. ALL NUTS AND WASHERS SHALL BE HOT DIP GALVANIZED TO GRADE G185 CONFORMING TO THE FOLLOWING ASTM STANDARDS AS APPLICABLE: ASTM A653. ASTM A123, AND ASTM A153.
- 5. ALL HARDWARE (HANGERS, NAILS, HEX BOLTS, CARRIAGE BOLTS, CONNECTORS, WASHERS, SCREWS, LAG SCREWS, ACCESSORIES, ETC.) SHALL BE HOT DIPPED GALVANIZED TO GRADE G185 CONFORMING TO ASTM F593.
- WOOD CONNECTOR HARDWARE SHALL CONFORM TO ANSI/ASME STANDARD B18.2.1. LAG SCREWS SHALL BE EMBEDDED A MINIMUM OF 8D (8* DIAMETER) AND MATCH THE DIAMETER INDICATED ON PLAN UNLESS NOTED OTHERWISE. THE CLEARANCE HOLE FOR THE SHANK SHALL HAVE THE SAME DIAMETER AS THE SHANK, AND THE SAME DEPTH OF PENETRATION AS THE LENGTH OF UNTHREADED SHANK. THE LEAD HOLE FOR THREADED PORTION SHALL HAVE A DIAMETER EQUAL TO 60% TO 75% OF THE SHANK DIAMETER AND A LENGTH EQUAL TO AT LEAST THE LENGTH OF THE THREADED PORTION. THE THREADED PORTION OF THE LAG SCREW SHALL BE INSERTED IN ITS LEAD HOLE BY TURNING WITH A LUBRICANT CAN BE USED ON THE LAG SCREW OR IN THE LEAD HOLES TO FACILITATE INSERTION AND TO PREVENT DAMAGE TO THE LAG SCREW. SEE PRESERVATIVE PRESSURE TREADED WOOD REQUIREMENTS FOR FASTENER REQUIREMENTS.

- 8. ALL BOLT HOLES IN WOOD MEMBERS SHALL BE MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN THE BOLT DIAMETER. PROVIDE PLATE WASHERS WHERE NUTS, BEAR ON WOOD. CUT WASHERS SHALL MEASURE 2 1/4"øx3/16" THICK.
- 9. WOOD CONNECTOR HARDWARE SHALL HAVE ICC APPROVAL AND BE MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, OR PRE-APPROVED EQUAL. PROVIDE MAXIMUM SIZE AND QUANTITY OF NAILS OR BOLTS PER MANUFACTURER, EXCEPT AS NOTED OTHERWISE. PROVIDE PRE-DRILLED HOLES TO PREVENT SPLITTING OF MEMBERS WHERE APPLICABLE.
- 10. WOOD SHALL BE PROTECTED FROM DECAY AND TERMITES IN ACCORDANCE WITH IBC 2304.12. PRESERVATIVE-TREATMENTS SHALL CONFORM TO THE APPROPRIATE STANDARDS OF THE AWPA FOR SAWN LUMBER AND GLUED-LAMINATED TIMBER AND SHALL BEAR A TREATMENT IDENTIFICATION MARK BY THE CERTIFYING AGENCY. ALL LUMBER SHALL BE PRESERVATIVE TREATED. PRESERVATIVE TREATMENT SHALL NOT REDUCE ALLOWABLE DESIGN STRESSES.
- 11. CUT AND DRILLED HOLES SHALL BE TREATED WITH COPPER NAPTHENATE BASED SOLUTION IN ACCORDANCE WITH AWPA STANDARD M4. THE PREERVATIVES CONCENTRATION SHALL CONTAIN NO LESS THAN 2% COPPER METAL. FIELD TREATMENT SHALL BE BY BRUSHING, DIPPING, OR SOAKING AND SHALL BE DONE IN SUCH A MANNER THAT THE PRESERVATIVES DO NOT DRIP OR SPILL ON THE GROUND OR IN THE WATER.

STRUCTURAL STEEL

REFERENCE SPECIFICATION:

STRUCTURAL STEEL AISC STEEL CONSTRUCTION MANUAL, 15TH EDITION

GALVANIZED CARBON STEEL:

ASTM A 36 PLATE ASTM F3125 ANCHOR BOLT ASTM A36 ANGLES TIE ROD BRACING ASTM A36 **ASTM F1145** TURNBUCKLES CLEVISES ASTM A668 ASTM A 36 THREADED ROD

STRUCTURAL STEEL DESIGN. FABRICATION AND ERECTION SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 22 OF THE INTERNATIONAL BUILDING CODE. SUBSTITUTION OF MEMBER SIZES OR STEEL GRADE WILL NOT BE ALLOWED WITHOUT PRIOR APPROVAL OF THE ENGINEER. BOLTED CONNECTIONS ARE TO BE ASTM A 325 BOLTS AS SHOWN. A MINIMUM OF TWO BOLTS IS REQUIRED FOR ALL BEAM CONNECTIONS. ALTERNATIVE CONNECTIONS TO THOSE SHOWN ON THESE DRAWINGS WILL REQUIRE PRIOR APPROVAL OF THE ENGINEER.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ERECTION AIDS AND JOINT PREPARATIONS THAT INCLUDE, BUT ARE NOT LIMITED TO, ERECTION ANGLES, LIFT HOLES AND OTHER AIDS, WELDING PROCEDURES, REQUIRED ROOT OPENINGS, ROOT FACE DIMENSIONS, GROOVE ANGLES, BACKING BARS, COPES, SURFACE ROUGHNESS VALUES, AND UNEQUAL PARTS.

HELWEINERS SHALL BE IN CONFORMANCE WITH AISC AND AWS STANDARDS, AND SHALL BE PERFORMED BY AWS-WABO-CERTIFIED WELDERS USING 70KSI ELECTRODES AND LOW HYDROGEN PROCESSES. ONLY WELDS THAT ARE PREQUALIFIED, AS DEFINED BY AWS, OR QUALIFIED BY TESTING SHALL BE USED. SHOP DRAWINGS SHALL SHOW ALL WELDING WITH AWS A2.4 SYMBOLS, WELDS SHOWN ON THE DRAWINGS ARE MINIMUM SIZES. INCREASE WELD SIZE TO AWS MINIMUM SIZES BASED ON PLATE THICKNESS. MINIMUM WELD SIZE SHALL BE 3/16-INCH, UNLESS NOTED OTHERWISE. THE WELDS SHOWN ARE FOR THE FINAL CONNECTIONS. FIELD WELD SYMBOLS ARE SHOWN WHERE FIELD WELDS ARE REQUIRED BY THE STRUCTURAL DESIGN. WHERE FIELD WELD IS NOT INDICATED, THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING IF A WELD SHOULD BE SHOP-OR FIELD-WELDED IN ORDER TO FACILITATE THE STRUCTURAL STEEL ERECTION. IF CONTRACTOR DETERMINES A FIELD-WELD NOT INDICATED ON DRAWINGS WILL FACILITATE STRUCTURAL STEEL ERECTION. SAID CHANGE SHALL BE INDICATED ON SHOP DRAWING SUBMITTAL FOR REVIEW.

GALVANIZING: ALL CARBON STEEL AND CONNECTIONS SHALL BE HOT-DIPPED GALVANIZED AFTER FABRICATION IN COMPLIANCE WITH ASTM A 123 GRADE 100 UNLESS NOTED OTHERWISE. ALL FIELD WELDS ON GALVANIZED MATERIAL SHALL BE REPAIRED IN ACCORDANCE WITH ASTM A780, METHOD A1 USING ZINC WELD STICK, COMPLYING WITH THE SPECIFICATIONS. ALL PLATES AND OTHER STEEL ITEMS EMBEDDED IN CONCRETE SHALL BE HOT-DIPPED GALVANIZED AFTER FABRICATION.

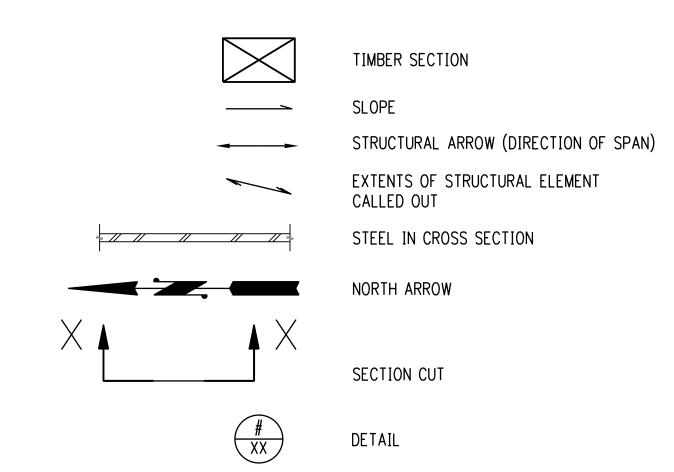
ANCHORS

USE OF DRILLED CONCRETE ADHESIVE ANCHORS, WHERE NOT SPECIFIED IN THE DOCUMENTS SHALL BE SUBJECT TO APPROVAL BY THE ENGINEER.

ADHESIVE ANCHORS:

EPOXY OR ADHESIVE-TYPE ANCHORS AND DOWELS SHALL BE INSTALLED PER THE MANUFACTURER'S RECOMMENDATION. SPECIAL INSPECTION IS REQUIRED FOR ALL ADHESIVE ANCHORS. ACCEPTABLE ADHESIVES ARE HILTI-HIT-RE 500 SD, POWERS PE 1000, SIMPSON SET-XP, OR BETTER, ICC REPORT SHALL BE SUBMITTED FOR ALL ADHESIVE ANCHOR PRODUCTS.

STRUCTURAL DRAWING SYMBOLS



ANCHOR BOLT ADDITIONAL BOTTOM OF BEAM BOTTOM BEARING BETTER BETWEEN CENTER TO CENTER CENTERLINE CLEAR CONCRETE CONNECTION CONSTRUCTION CONSTRUCTION CONTINUOUS CONTRACTOR CENTER DETAIL DIAMETER DIAGONAL DIMENSION DRAWING EACH EACH FACE ELEVATION EMBEDMENT EQUAL EXISTING FEET GALVANIZED GENERAL CRIDILINE	K KSF KSI LF LLV MAX MIN MISC NDT NTS OC PL PSI PSF REQ'D SECT SHT SIM SQ STL STRUCT SYM T/ T&B TYP UNO VERT W/	KIP (1,000 LBS) KIPS PER SQ. FT. KIPS PER SQ. INCH LINEAL FOOT LONG LEG HORIZONTAL LONG LEG VERTICAL MAXIMUM MINIMUM MISCELLANEOUS NON DESTRUCTIVE TESTING NOT TO SCALE ON CENTER PLATE POUNDS PER SQ. IN. POUNDS PER SQ. FT. REFERENCE DIMENSION REQUIRED SECTION SHEET SIMILAR SQUARE STEEL STRUCTURAL SYMMETRICAL TOP OF TOP AND BOTTOM TYPICAL UNLESS NOTED OTHERWISE VERTICAL WITH
GENERAL GRIDLINE HORIZONTAL INTERNATIONAL BUILDING CODE INSIDE DIAMETER INCH	W/ WHS W/O WP WT	WITH WELDED HEADED STUD WITHOUT WORK POINT STRUCTURAL TEE
	ADDITIONAL BOTTOM OF BEAM BOTTOM BEARING BETTER BETWEEN CENTER TO CENTER CENTERLINE CLEAR CONCRETE CONNECTION CONSTRUCTION CONTINUOUS CONTRACTOR CENTER DETAIL DIAMETER DIAMETER DIAGONAL DIMENSION DRAWING EACH EACH FACE ELEVATION EMBEDMENT EQUAL EXISTING FEET GALVANIZED GENERAL GRIDLINE HORIZONTAL INTERNATIONAL BUILDING CODE INSIDE DIAMETER	ADDITIONAL BOTTOM OF BEAM BOTTOM BEARING BETTER BETWEEN CENTER TO CENTER CONCRETE CONNECTION CONSTRUCTION CONSTRUCTION CONTINUOUS CONTRACTOR CENTER DETAIL DIAMETER DETAIL DIAMETER DIAGONAL DIMENSION DRAWING EACH EACH EACH EACH EACH EACH EACH GRIDLINE CLEAR CONTRACTOR COSTRUCTION CONSTRUCTION CONTRUCTION C

STRUCTURAL DRAWING ABBREVIATIONS

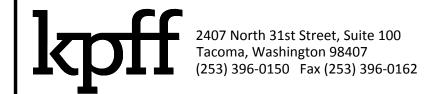
SPECIAL INSPECTION

SPECIAL INSPE			
ESTABLISHED PER 2015 IBC	C SECTION 10	9 & CHAPTE	ER 17
ITEM	CONTINUOUS INSPECTION	l	COMMENTS
PREFAB CONSTRUCTION			REF. NOTES 2, 8
THE NE CONSTRUCTION			NEI: 110123 2, 0
CONCRETE			
ADHESIVE ANCHOR INSTALLATION	X		REF. NOTE 5
STRUCTURAL STEEL			
FABRICATION & ERECTION		Χ	REF. NOTE 6
SINGLE PASS FILLET WELDS ≤ 5/16"		Χ	REF. NOTE 7
FILLET WELDS > 5/16"	X		REF. NOTE 7
PARTIAL JOINT PENETRATION WELDS	X		
TIMBER			
NAILING AND BOLTING MEMBERS		Х	
PILES			
PILE DRIVING	X		BY GEOTECH

INSPECTION SCHEDULE NOTES:

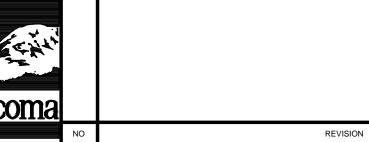
- THE ITEMS CHECKED WITH AN "X" SHALL BE INSPECTED IN ACCORDANCE WITH IBC CHAPTER 17 BY A CERTIFIED SPECIAL INSPECTOR FROM AN ESTABLISHED TESTING AGENCY. FOR MATERIAL SAMPLING AND TESTING REQUIREMENTS, REFER TO PROJECT SPECIFICATIONS, THE STRUCTURAL NOTES AND THE NOTES BELOW. THE TESTING AGENCY SHALL SEND COPIES OF ALL STRUCTURAL TESTING AND INSPECTION REPORTS DIRECTLY TO THE ENGINEER, CONTRACTOR AND WDFW. ANY MATERIALS WHICH FAIL TO MEET THE PROJECT SPECIFICATIONS SHALL IMMEDIATELY BE BROUGHT TO THE ATTENTION OF THE ENGINEER. SPECIAL INSPECTION TESTING REQUIREMENTS APPLY EQUALLY TO ALL BIDDER DESIGNED COMPONENTS.
- 2. SPECIAL INSPECTION IS NOT REQUIRED FOR WORK PREFORMED BY AN AISC CERTIFIED FABRICATOR PER IBC SECTION 1704.2.5.2.
- 3. CONTINUOUS SPECIAL INSPECTION MEANS THAT THE SPECIAL INSPECTOR IS ON THE SITE AT ALL TIMES OBSERVING THE WORK REQUIRING SPECIAL INSPECTION (IBC 1702). PERIODIC SPECIAL INSPECTION MEANS THAT THE SPECIAL INSPECTOR IS ON SITE AT TIME INTERVALS NECESSARY TO CONFIRM THAT ALL WORK REQUIRING SPECIAL INSPECTION IS IN COMPLIANCE.
- 4. INSPECTION REQUIREMENTS FOR SYSTEMS DESIGNED BY OTHERS SHALL BE DEFINED BY THE REGISTERED DESIGN PROFESSIONAL RESPONSIBLE FOR THEIR DESIGN.
- 5. INSPECTION OF DRILLED CONCRETE ANCHORS, INCLUDING ADHESIVE ANCHORS, SHALL INCLUDE VISUAL VERIFICATION OF DRILLED HOLE DEPTH, SPACING, EDGE DISTANCES AND HOLE CLEANING. GROUT INSTALLATION SHALL BE OBSERVED AND GROUT PRODUCT SPECIFICATION AND PREPARATION SHALL BE VERIFIED.
- 6. INSPECTION OF STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH IBC SECTION 1705.2.
- 7. ALL WELDS SHALL BE VISUALLY INSPECTED.
- 8. INSPECTION FOR PREFABRICATED CONSTRUCTION SHALL BE THE SAME AS IF THE MATERIAL USED IN THE CONSTRUCTION TOOK PLACE ON SITE. CONTINUOUS INSPECTION WILL NOT BE REQUIRED DURING PREFABRICATION IF THE APPROVED AGENCY CERTIFIES THE CONSTRUCTION AND FURNISHES EVIDENCE OF COMPLIANCE.

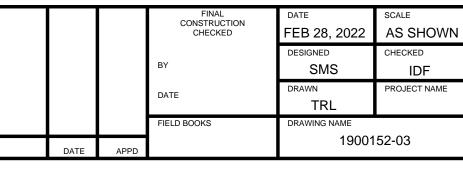
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DocuSigned by: Clinis Carson

CITY OF TACOMA DEPARTMENT OF PUBLIC WORKS

15TH ST TRANSIENT MOORAGE DOCK REPLACEMENT STRUCTURAL NOTES

PW-00000-00-0² ET NO. S3 03 _{OF} 14

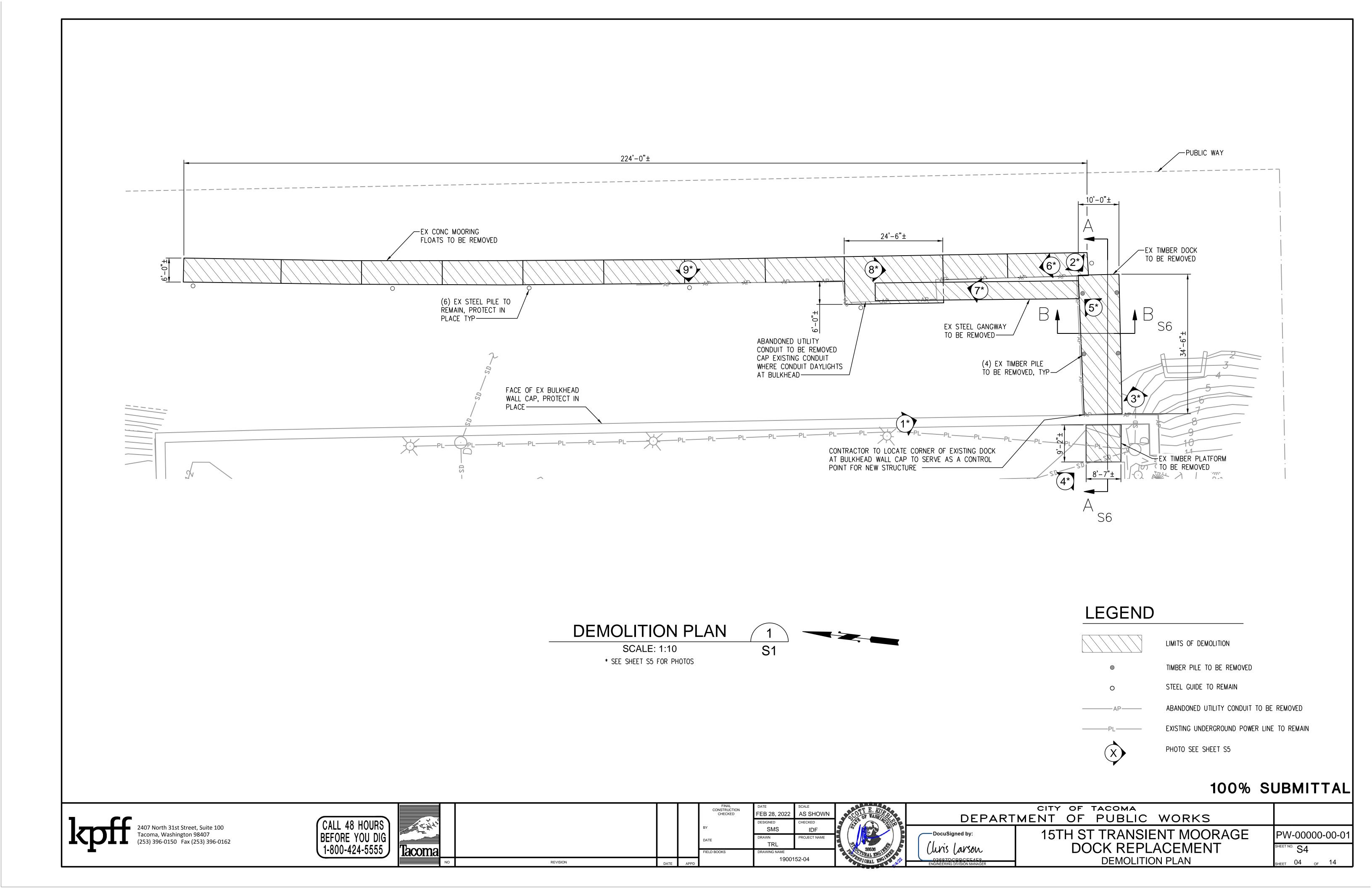




PHOTO 1 EXISTING TIMBER DOCK - 1



PHOTO 4 EXISTING PLATFORM



PHOTO 7 EXISTING FLOATS - 1

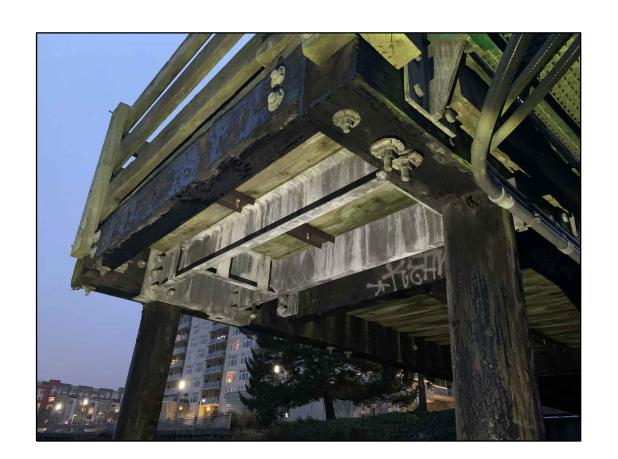


PHOTO 2 EXISTING TIMBER DOCK - 2



PHOTO 5 **EXISTING GANGWAY - 1**



PHOTO 8 EXISTING FLOATS - 2



PHOTO 3 **EXISTING TIMBER DOCK - 3**

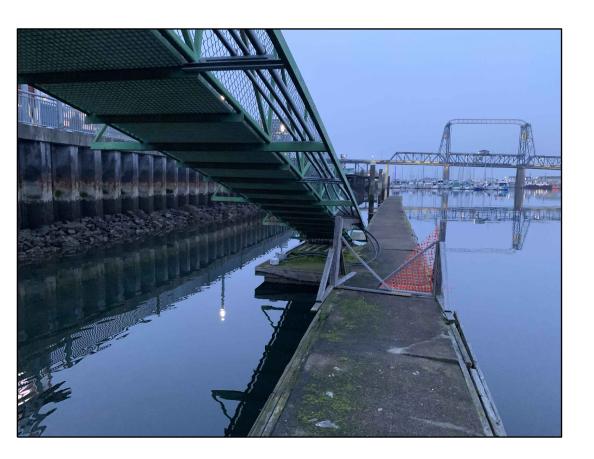


PHOTO 6 EXISTING GANGWAY - 2

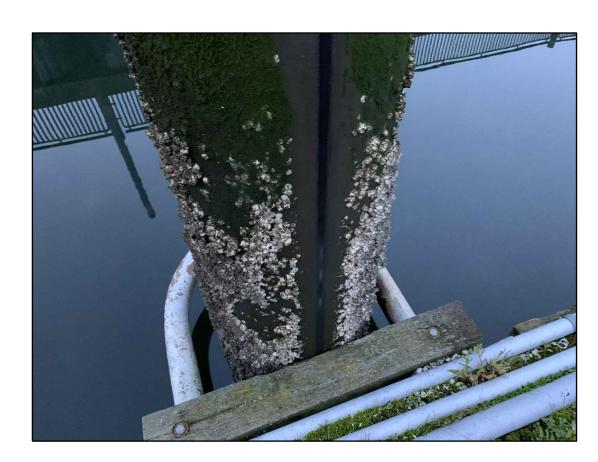
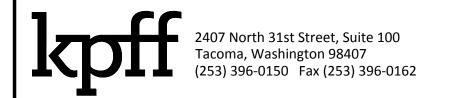
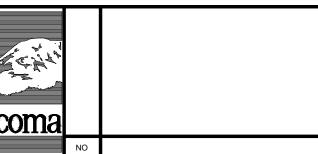


PHOTO 9 EXISTING FLOATS - 3 100% SUBMITTAL









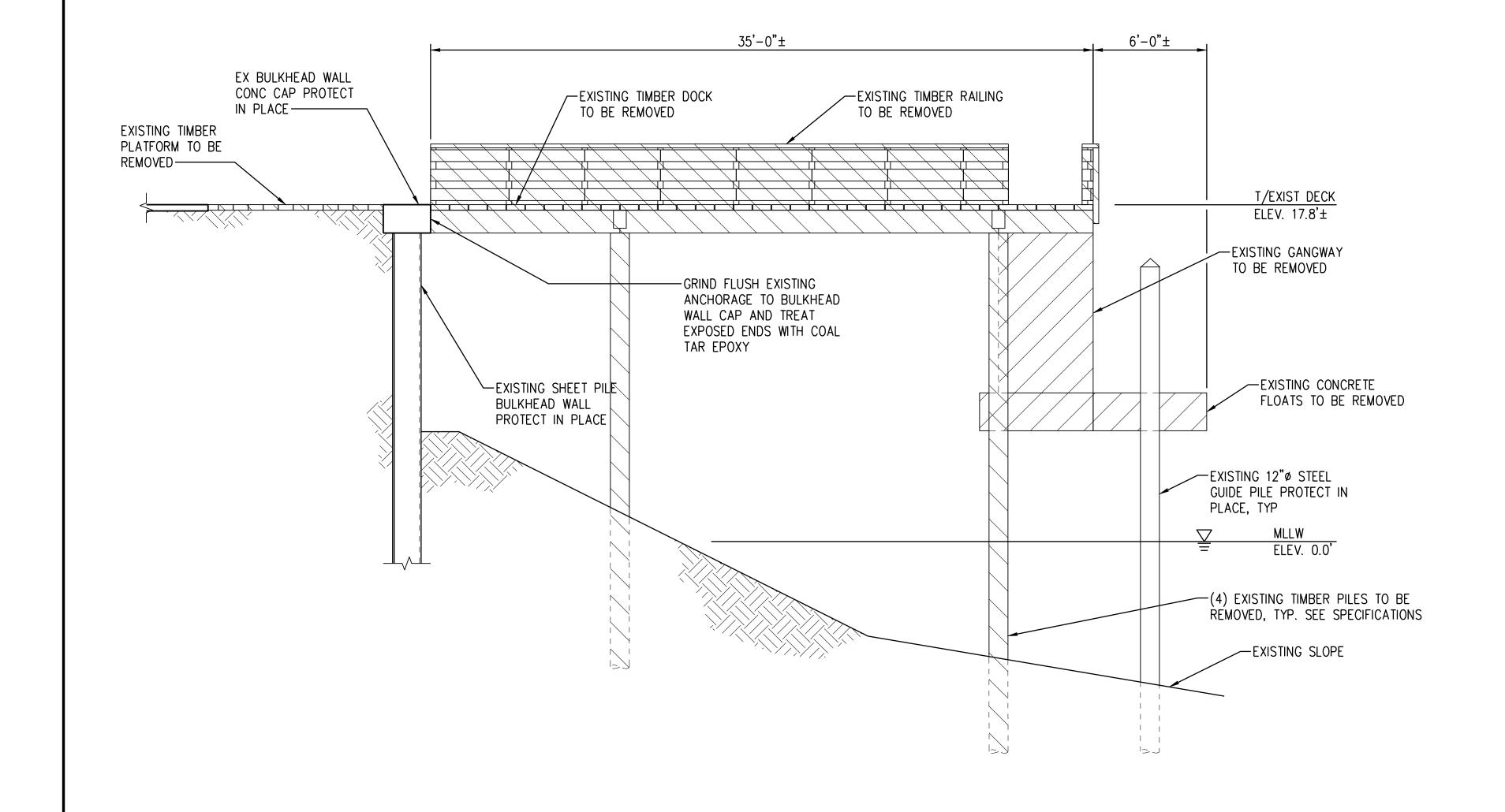


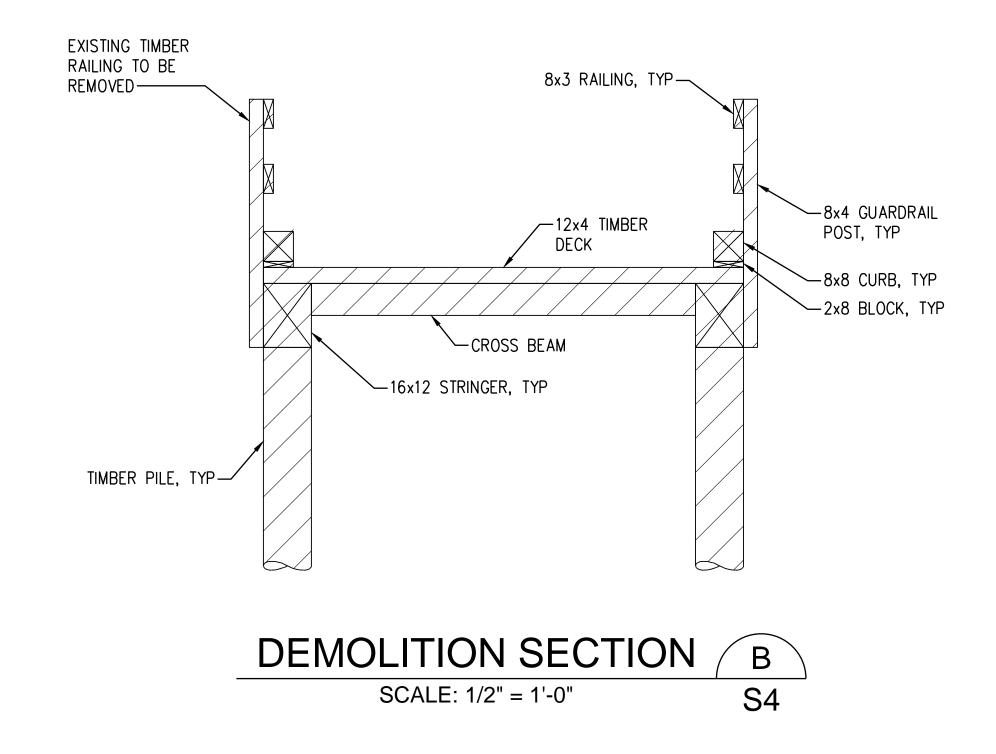
Chris Larson

CITY OF TACOMA
DEPARTMENT OF PUBLIC WORKS

15TH ST TRANSIENT MOORAGE DOCK REPLACEMENT EXISTING CONDITION PHOTOS

PW-00000-00-01





DEMOLITION SECTION SCALE: 1/4" = 1'-0" S4

LEGEND

LIMITS OF DEMOLITION

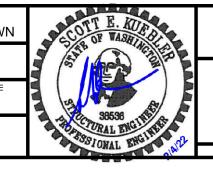
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CALL 48 HOURS BEFORE YOU DIG 1-800-424-5555



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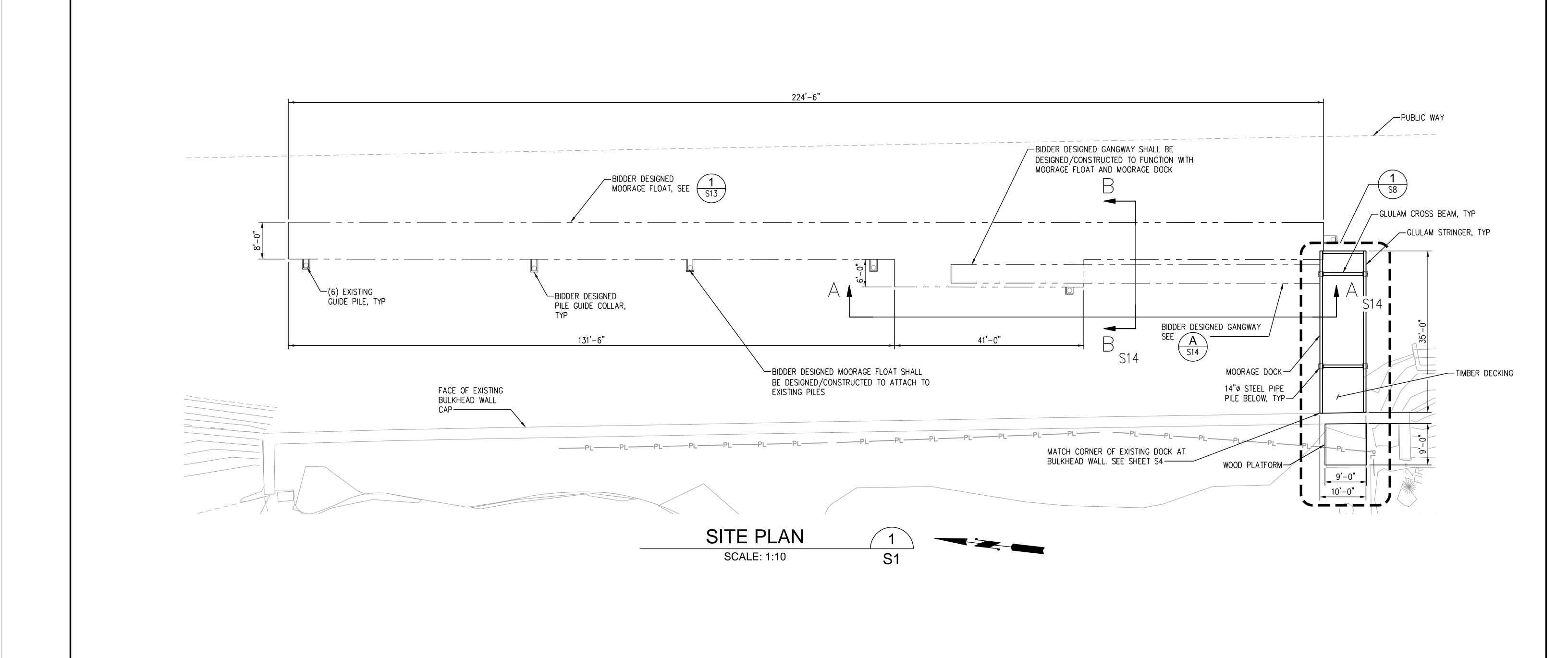
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DocuSigned by: Chris Larson

CITY OF TACOMA
DEPARTMENT OF PUBLIC WORKS 15TH ST TRANSIENT MOORAGE DOCK REPLACEMENT DEMOLITION DETAILS

PW-00000-00-0² S6



FINAL CONSTRUCTION CHECKED

100% SUBMITTAL



CALL 48 HOURS BEFORE YOU DIG 1-800-424-5555



Tacoma			
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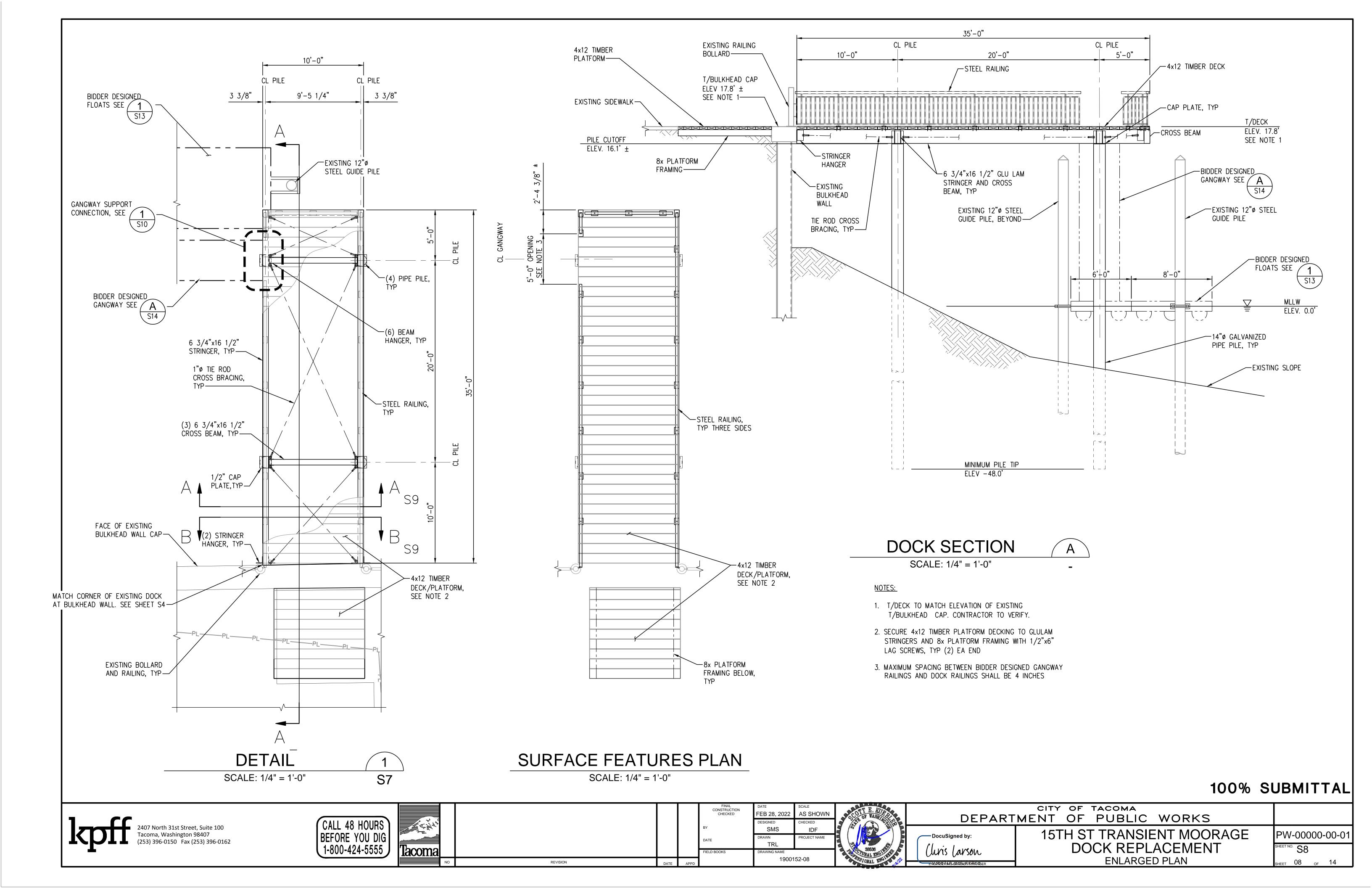
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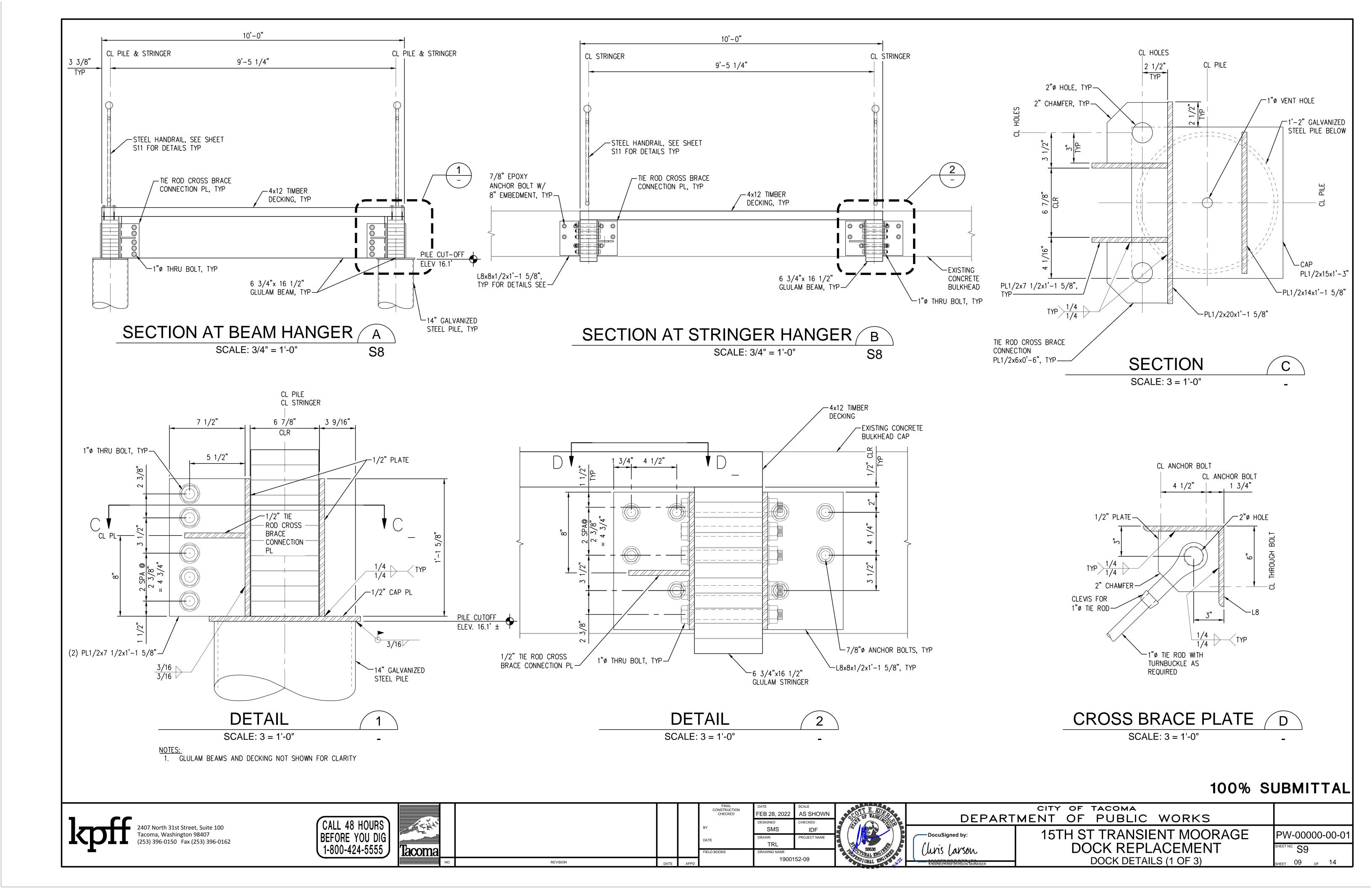
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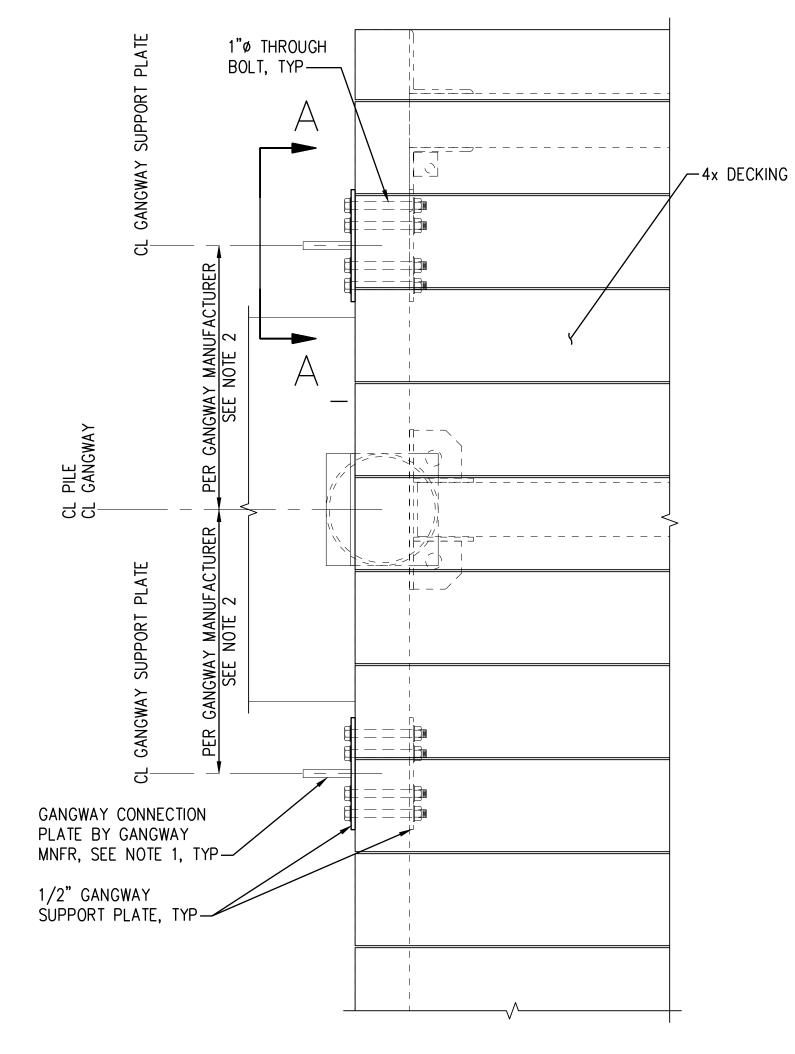
CITY OF TACOMA DEPARTMENT OF PUBLIC WORKS —DocuSigned by: Cliris Larson

15TH ST TRANSIENT MOORAGE DOCK REPLACEMENT SITE PLAN

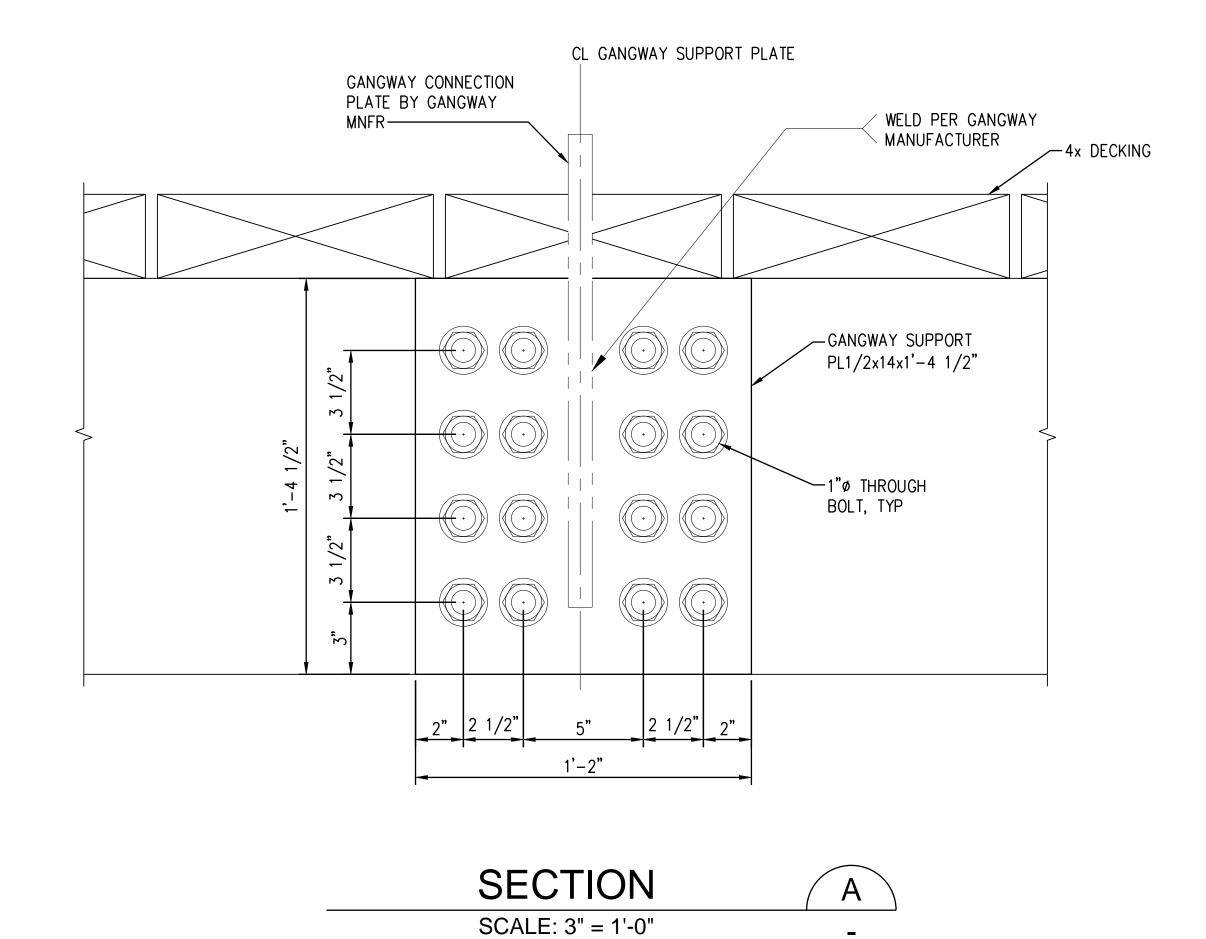
PW-00000-00-01 HEET NO. S7 ET 07 OF 14











NOTE:

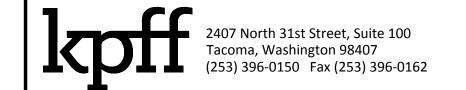
1. THIS ASSEMBLY WAS DESIGNED FOR THE FOLLOWING MAXIMUM SERVICE LEVEL REACTIONS FROM THE GANGWAY, USING LOAD COMBINATIONS IN ACCORDANCE WITH THE 2018 IBC:

 $DEAD\ LOAD\ =\ 3.5K$ LIVE LOAD = 20K SNOW LOAD = 20KHORIZ WIND LOAD = 3KHORIZ SEISMIC LOAD = 3.5K

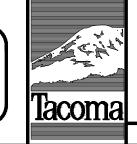
CONTRACTOR SHALL VERIFY WITH THE ENGINEER THAT THESE ASSUMED REACTIONS ARE NOT EXCEEDED. REDESIGN MAY BE REQUIRED IF THESE LOADS ARE EXCEEDED.

2. CONTRACTOR SHALL COORDINATE WITH THE ENGINEER TO VERIFY GEOMETRY OF GANGWAY SUPPORT ASSEMBLY.

100% SUBMITTAL



CALL 48 HOURS BEFORE YOU DIG 1-800-424-5555





FINAL CONSTRUCTION CHECKED SMS IDF TRL 1900152-10

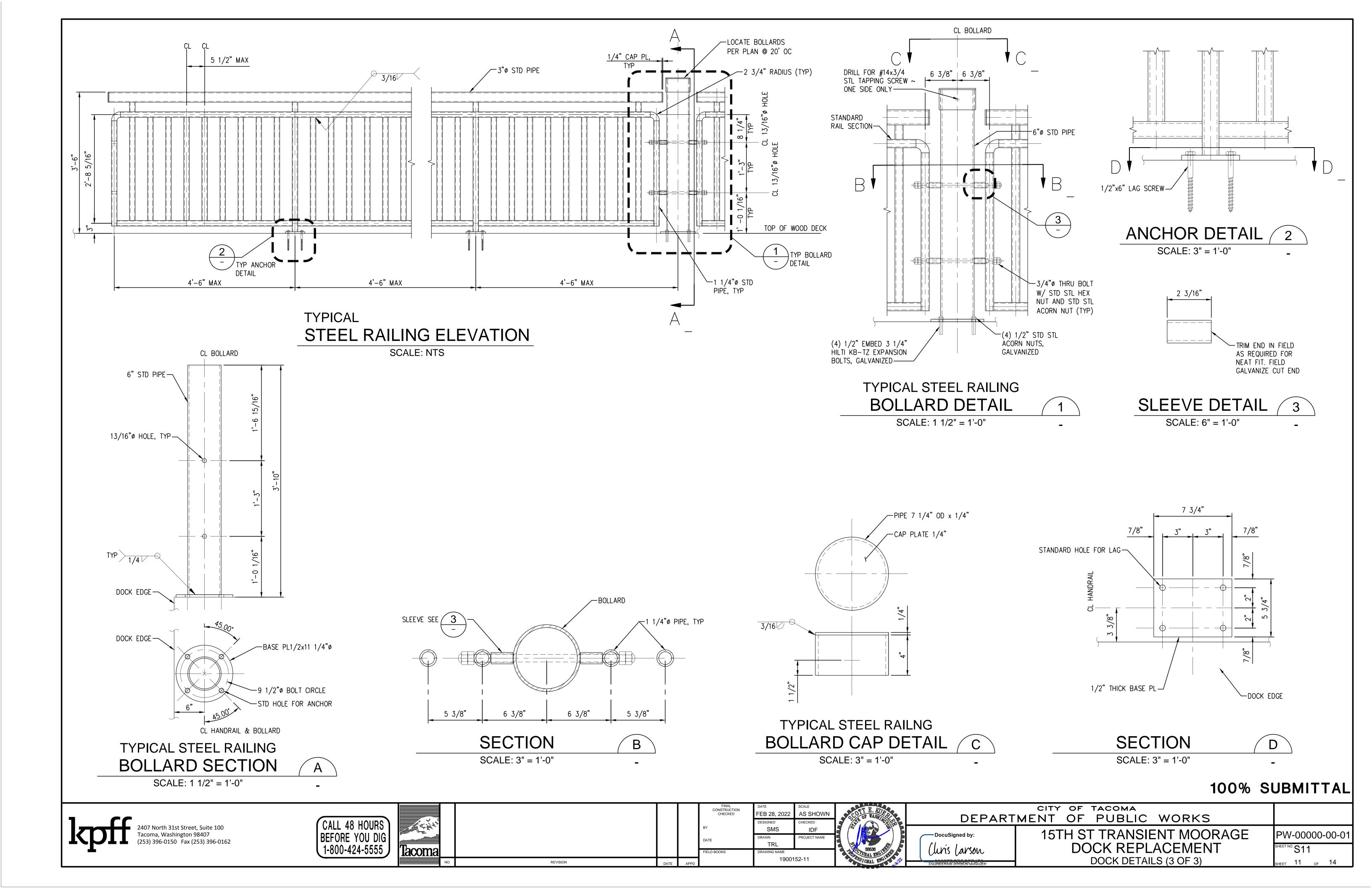


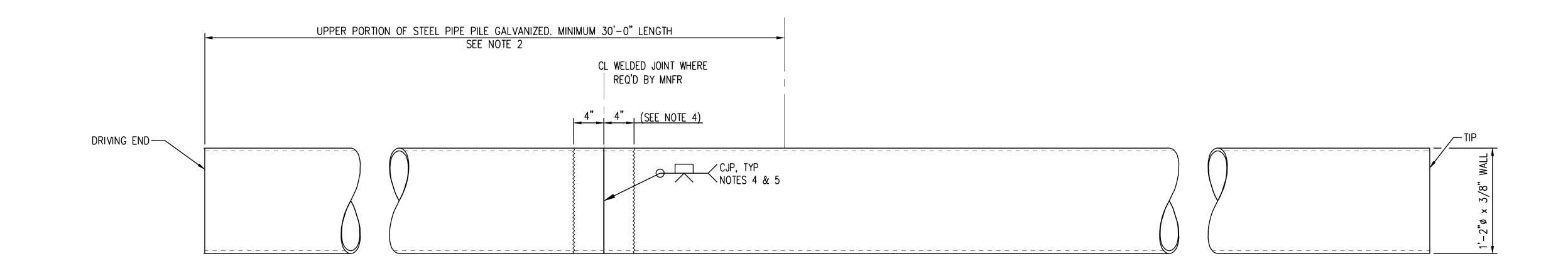
DEPART	MENT	OF	PUBLIC	WORKS
—Docusigned by:	157	_		ENT MOOF ACEMEN

15TH ST TRANSIENT MOORAGE DOCK REPLACEMENT DOCK DETAILS (2 OF 3)

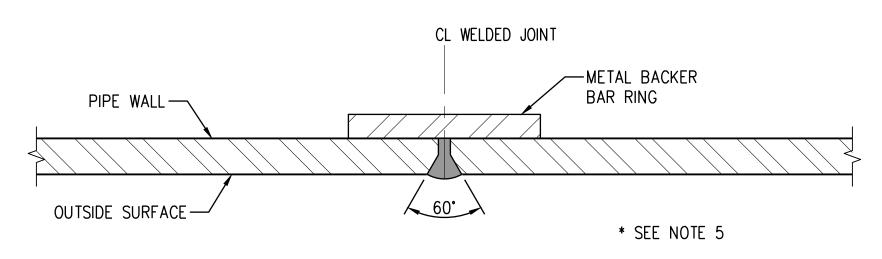
CITY OF TACOMA

PW-00000-00-01 S10 10 _{OF} 14





SCALE: 1 1/2" = 1'-0"



CJP TUBULAR BUTT JOINT SCALE: 12" = 1'-0"

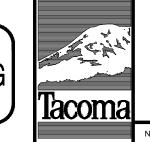
NOTES:

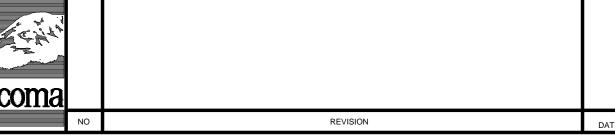
- 1. ALL STEEL PILES SHALL CONFORM TO ASTM A252, GRADE 3, WITH MODIFICATIONS AS INDOCATED IN THE SPECIFICATIONS.
- 2. UPPER PORTION OF STEEL PILE TO BE HOT-DIP GALVANIZED IN ACCORDANCE WITH ASTM A123 THICKNESS GRADE 100.
- 3. WELDING SHALL CONFORM TO "STRUCTURAL WELDING CODE STEEL", AWS D1.1
- 4. HOLD BACK OR REMOVE GALVANIZED COATING FROM CENTERLINE WELDED JOINT ON INSIDE AND OUTSIDE OF PIPE BEFORE WELDING. AFTER JOINT IS WELDED, APPLY GALVANIZING PER SPECIFICATIONS FOR REPAIR OF GALVANIZED COATING.
- 5. ALL CJP TUBULAR BUTT JOINTS SHALL HAVE A WPS QUALIFIED IN ACCORDANCE WITH AWS D1.1 SECTION 4.
- 6. FOR PILE CUTOFF ELEVATION, SEE SHEETS S8 AND S9. MINIMUM PILE TIP ELEVATION SHALL BE -48'. SEE SPECIFICATIONS.
- 7. MINIMUM PILE ORDER LENGTH SHALL EQUAL PILE CUTOFF ELEVATION MINUS PILE TIP ELEVATION PLUS 5 FT OVERDRIVE ALLOWANCE.

100% SUBMITTAL



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FINAL CONSTRUCTION CHECKED IDF TRL 1900152-12

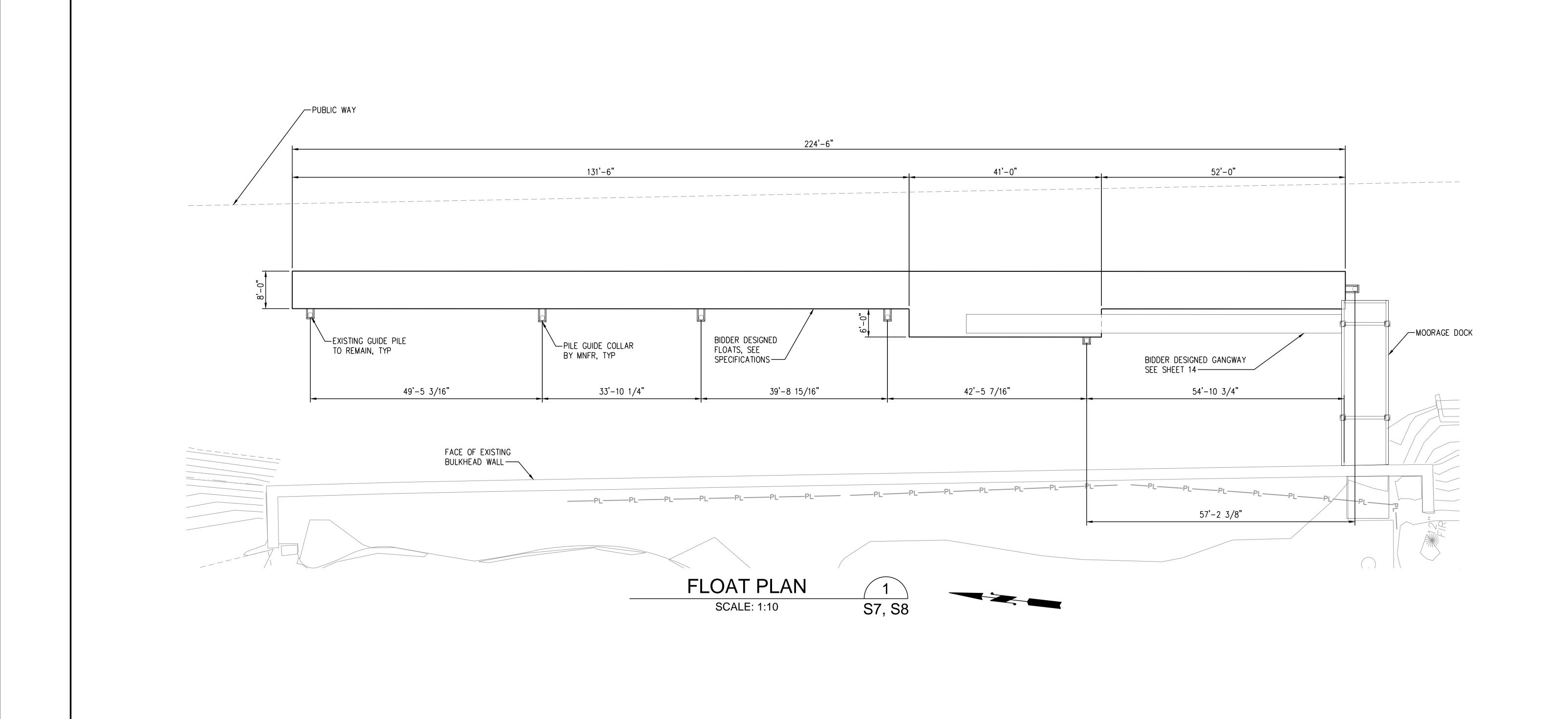


DEPARTMENT OF PUBLIC WORKS Chris Larson

15TH ST TRANSIENT MOORAGE DOCK REPLACEMENT STEEL PILE DETAILS

CITY OF TACOMA

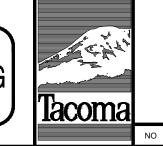
PW-00000-00-01 S12 T 12 OF 14



100% SUBMITTAL



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Tacoma		
NO	REVISION	DATE

	CONSTRUCTION CHECKED	FEB 28, 2022	AS SHOWN	
		DESIGNED	CHECKED	4
	BY	SMS	IDF	3
	DATE	DRAWN	PROJECT NAME	4
	5/112	TRL		3
	FIELD BOOKS	DRAWING NAME		4
APPD		1900152-13		



CITY OF TACOMA
DEPARTMENT OF PUBLIC WORKS 15TH ST TRANSIENT MOORAGE DOCK REPLACEMENT FLOAT DETAILS Cluris Larson

PW-00000-00-01 HEET NO. S13

